# I-375 EXPANDED STUDY AREA ANALYSIS TECHNICAL MEMORANDUM

June 2, 2020



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#### 1 Introduction

This memorandum is intended to document the traffic analysis process and recommendations of the expanded study area after the dynamic traffic assignment (DTA) modeling was completed.

The removal of the Gratiot Avenue Connector and conversion of I-375 from a freeway to a boulevard, along with other associated improvements, would result in vehicular rerouting that would be widely dispersed. However, there are some key corridors that may exhibit a more concentrated increase in volumes as a result. The intent of the expanded study area analysis is to identify alternate route corridors within the central business district (CBD) that may experience more traffic as a result of the conversion and analyze those locations using Synchro or Highway Capacity Software (HCS). The analysis results will confirm that the locations can support the increased volume or will provide recommendations for improvements if the volumes can't be supported. If large improvements are shown to be needed, additional assumptions in the dynamic traffic assignment model may be needed.

Per the I-375 Design Criteria, Level of Service (LOS) D is considered acceptable for the city grid, while LOS C is acceptable for the freeway and system ramps. The term "acceptable" in this document refers to these standards.

This analysis was conducted in the expanded study area to supplement Vissim modeling that was used for analysis of the primary study area, as documented in *I-375 Vissim Methods and Assumptions*.

# 2 Recommended Approach

The maps provided in Appendix A highlight any segments in green that may have an average increase of 200 vehicles per hour (vph) or more during the peak period as a result of the project. A baseline of 200 vph was chosen because it brings to light a subset of reroutes that are most likely to be taken. This 200 vph value was developed as 10% of the average saturation flow rate of the DTA model, which was approximately 2,000 vehicles per hour per lane (vphpl). Increases of fewer than 200 vph are unlikely to show significant changes in level of service. Note that some segments highlighted in green can be attributed to "modeling noise", which indicate unexplained volume increases that are unlikely to be directly related to the proposed changes. The study team analyzed all corridors within the study area to determine whether information presented was modeling noise or vehicular reroutes caused by the project.

The DTA model was also used to determine the corridors that could see delay of 10 seconds or more. Nearly all of these segments are contained within either the primary or expanded study area.

Based on the DTA modeling effort, the study team analyzed several intersections and freeway segments. The limits of the expanded study area are documented in the following sections.

#### 2.1 HCS Analysis Recommendation

The following corridor was analyzed using HCS as part of the expanded study area analysis.

#### M-10 between Martin Luther King Blvd and Jefferson Avenue

The DTA model showed a potential increase in diverted traffic along M-10 in the Preferred Alternative in both the AM and PM conditions. The limits for the analysis included both northbound and southbound M-10 from Martin Luther King Blvd, north of I-75, and continue south until M-10 becomes Jefferson Avenue.

There are a total of 18 HCS segments that were analyzed in the northbound and southbound segments along M-10.

#### 2.2 Synchro Analysis Recommendation

Signalized intersections were analyzed in the following corridors based on the DTA analysis:

- 1. Brush Street from I-75 to Jefferson Avenue
- 2. Mack Avenue from I-375 to St. Aubin Street
- 3. Randolph Street from Gratiot Avenue to Jefferson Avenue
- 4. Beaubien Street. from Gratiot Avenue to Jefferson Avenue
- 5. Congress Street from M-10 to Beaubien Street
- 6. Woodward Avenue from Montcalm Street to Gratiot Avenue

There are 34 signalized intersections that were analyzed within these corridors.

#### 3 Data Collection

The following data was collected to conduct the analysis of the expanded study area.

#### 3.1 Traffic Counts

Counts were collected from a variety of data sources.

Vehicle classification and turning movement counts were collected in October and November 2016 at the following locations:

- 1. Beaubien St. & Gratiot Ave.
- 2. Congress St. & First St.
- 3. Congress St. & Griswold St.
- 4. Congress St. & Shelby St.
- 5. Congress St. & Washington Blvd.
- 6. Gratiot Ave. & Brush St.

- 7. Jefferson Ave. & Griswold St.
- 8. Randolph St. & Cadillac Square North
- 9. Randolph St. & Cadillac Square South
- 10. Randolph St. & Congress St.

- 11. Randolph St. & Gratiot Ave.
- 12. Randolph St. & Lafayette Ave.
- 13. Randolph St. & Larned St.
- 14. Randolph St. & Monroe Ave.

Additional vehicle classification and turning movement counts were collected in September 2018 at the following locations:

- 1. Beaubien St. & Congress St.
- 2. Beaubien St. & Fort St.
- 3. Beaubien St. & Lafayette Ave.
- 4. Beaubien St. & Larned St.
- 5. Beaubien St. & Monroe Ave.
- 6. Brush St. & Adams St.
- 7. Brush St. & Beacon St.
- 8. Brush St. & Congress St.

- 9. Brush St.& Larned St.
- 10. Brush St. & Madison Ave.
- 11. Brush St. & Monroe Ave.
- 12. Brush St. & Montcalm St.
- 13. Mack Ave. & Russell St.
- 14. Congress St. & Woodward Ave.
- 15. Congress St. & Bates St.
- 16. Mack Ave. & St Aubin St.

Additional vehicle classification and turning movement counts were collected in October 2018 at the following locations along Woodward Ave.:

- 1. Woodward Ave. & Montcalm St.
- 2. Woodward Ave. & Elizabeth St.
- 3. Woodward Ave. & Adams Ave.
- 4. Woodward Ave. & Park Ave.
- 5. Woodward Ave. & John R. St.
- 6. Woodward Ave. & Grand River Ave.
- 7. Woodward Ave. & Gratiot Ave.

Freeway and ramp counts, in 15-minute intervals, were taken on various dates in 2015 and 2016 on the freeway and the collector/distributor (C/D) road.

#### 3.2 Signal Data

Existing signal timing data was collected from 2014 signal timing permits and a 2017 study. The study from 2017 included pre-developed Synchro models that were used in the analysis. The seven signalized intersections along the Woodward Avenue corridor were not included in the previous developed Synchro models. The signal timing permits were not utilized for the Woodward Avenue signal timings but were optimized based on existing and future traffic volumes. Not including the timing permits for Woodward Avenue was determined to be acceptable since the LOS comparisons for the corridor are against the No-Build and Build optimized signal timings and not the current signal timings. All pedestrian facilities at the traffic signals along Woodward Avenue were accounted for in the signal timings.

Signal data from 2014 was collected at the following locations:

- 1. Congress St. & First St.
- 2. Congress St. & Cass St.
- 3. Congress St. & Washington Blvd.
- 4. Congress St. & Shelby St.
- 5. Congress St. & Griswold St.
- 6. Congress St. & Woodward Ave.

7. Congress St. & Bates St.	13. Beaubien St. & Congress St.
8. Brush St. & Larned St.	14.Beaubien St. & Fort St.
9. Brush St. & Congress St.	15. Beaubien St. & Lafayette Ave.
10. Brush St. & Monroe Ave.	16 Beaubien St. & Monroe Ave.
11.Brush St. & Madison Ave.	17. Mack & Russell St.
12.Beaubien St. & Larned St.	18.Mack & St Aubin St.

#### Data from 2017 was collected at the following locations:

12. Randolph St. & Monroe Ave.
13. Randolph St. & Gratiot Ave.
14.Brush St. & Larned St.
15. Brush St. & Congress St.
16. Brush St. & Monroe Ave.
17. Brush St. & Gratiot Ave.
18. Brush St. & Madison Ave.
19. Beaubien St. & Larned St.
20. Beaubien St. & Congress St.
21. Beaubien St. & Gratiot Ave.

Where signal data was available from both the 2014 and 2017 sources, the 2017 sources took precedent.

# 4 Synchro Development Methodology

## 4.1 Develop Existing Models

An existing condition Synchro model was created to use as a baseline for the Future No-Build (FNB) and Preferred Alternative scenarios. Synchro files were obtained from MDOT and existing roadway geometry was verified. Signal timing permits were also obtained and the existing signal timings were entered into the Synchro model. The seven traffic signals along Woodward Avenue from Montcalm Street to Gratiot Avenue were added to the Synchro files obtained from MDOT due to Woodward Avenue being an alternate route under the Preferred Alternative. Once traffic counts were completed, volumes for the study area were inputted into AM and PM versions of the Synchro model.

# 4.2 Develop Future No-Build Models

The FNB used the same geometry as the existing condition but applied a growth factor the volumes. The FNB scenario used a growth factor of 0.5% per year compounded from year 2017 to 2040. Signal timings were optimized to better suit these higher volumes.

### 4.3 Develop Future Preferred Alternative Models

The Preferred Alternative used proposed geometry and forecasted traffic volumes (see the *I-375 Traffic Forecasting Methodology Technical Memorandum* for further details). Similar to the FNB scenario, signal timings were optimized to suit the new volumes.

The main geometric changes implemented in the Synchro Preferred Alternative scenario were the conversion of select streets from one-way to two-way. The roads converted from one-way to two-way are as follows:

- Lafayette Avenue one lane in each direction from Randolph Street to Beaubien Street
- Fort Street one lane in each direction from Randolph Street to Brush Street
- Brush Street one lane in each direction from Congress Street to Gratiot Avenue
- Beaubien Street one lane each direction from Clinton Street to Madison Avenue

# 5 Synchro Results

Section 2 of this document identified a Synchro study area containing 34 signalized intersections. High-level results for these intersections can be found in Figure 1. Detailed results can be found in Table 5-1 through Table 5-6 summarizes the levels of service for the Woodward Avenue corridor for the existing conditions, Future No-Build, and Preferred Alternative. The delay breakdown by movement is included in Appendix B for the signalized intersections.

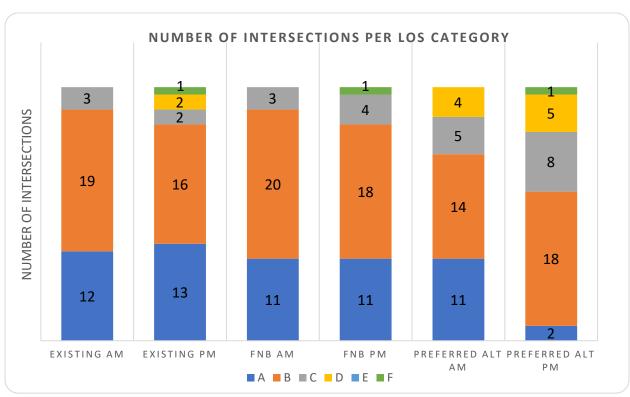


Figure 1: Number of Intersections per LOS Category

Figure 1 shows that the majority of the intersections across all scenarios would be LOS A-D. The AM and PM Preferred Alternative would have the majority of their intersections

with LOS A-D, but a few intersections may have a LOS E-F. This is partially due to forecasted increase in traffic volume and to the change in I-375 from a freeway to a surface street – there are more options for traffic to divert to other routes, increasing traffic at intersections in the nearby area.

Table 5-1 through Table 5-6 summarizes the levels of service for the Woodward Avenue corridor for the existing conditions, Future No-Build, and Preferred Alternative. Table 5-6 detail the LOS for each intersection within each scenario. The tables are split by corridor.

#### 5.1 Brush Street Corridor

Table 5-1: Brush Street Corridor LOS

Intersection	Exis	sting	Future N	No-Build	Preferred Alternative		
	AM	PM	AM	PM	AM	PM	
Brush/Adams	Α	Α	Α	Α	Α	В	
Brush/Beacon	Α	Α	Α	Α	Α	Α	
Brush/Congress	В	В	В	В	В	С	
Brush/Larned	В	В	В	В	С	D	
Brush/Madison	В	В	В	В	В	В	
Brush/Monroe	В	В	В	В	В	В	
Brush/Gratiot	В	В	В	В	D	С	

All intersections along the Brush Street corridor have a LOS of D or greater for all scenarios except for the AM Preferred Alternative scenario at the Brush Street and Gratiot Avenue. The intersection is expected to be a LOS E in large part because of the traffic volume making a westbound left turn from Gratiot Avenue to Brush Street. The AM Preferred Alternative scenario shows 211 vehicles making the left turn and may cause operational issues at the intersection. The pavement markings and signage indicate left turns are not allowed. The operations at the intersection will improve to LOS D if a left turn lane with storage is constructed to provide protection for the westbound left turn from Gratiot Avenue to Brush Street. The build results assume the left turn storage lane on the westbound approach.

Currently, Congress Street between Brush Street and Beaubien Street is one-way and has a four-lane cross section, with two travel lanes and two parking lanes. When analyzing this configuration, the intersection of Brush St. & Congress St. operates at LOS F. In order to improve the PM Build scenario LOS of the Brush St./Congress St. intersection from LOS F to an acceptable level, it was assumed to turn one parking lane into a travel lane, creating three travel lanes and only one parking lane along Congress St. from Beaubien St. to Brush St. The changes to the westbound approach were incorporated into the Synchro model and the intersection results are reflected in Table

5-1. The traffic should be monitored in the future as traffic may utilize nearby routes and changing the parking lane into a westbound travel lane may not be necessary in the PM peak period.

#### 5.2 Mack Avenue Corridor

Table 5-2 summarizes the levels of service for the Mack Avenue corridor for the existing conditions, Future No-Build, and Preferred Alternative. All intersections along Mack Avenue either have or are expected to have a LOS of D or better for all scenarios.

Table 5-2: Mack Avenue Corridor LOS

Interception	Existing		Future N	lo-Build	Preferred Alternative		
Intersection	AM	PM	AM	PM	AM	PM	
Mack/St. Aubin	В	В	В	В	D	D	
Mack/Russell	Α	Α	В	Α	В	В	

While the intersection of Mack Avenue at St. Aubin Street is expected to have a LOS D in the AM and PM peak hour with the Preferred Alternative, the northbound approach is expected to have a LOS F in the AM peak hour, with a delay of 117.1 seconds. Currently, the northbound approach is one wide lane and the curb-to-curb width of the northbound approach is approximately 30 feet. Installing a dedicated left turn bay for the northbound left movement would improve the level of service for the northbound approach, however, there are bike lanes on the north leg of the intersection which may make installing a northbound left-turn lane infeasible. Also, a small storage lane for the westbound right turn at the intersection would improve operations based on the turning movement volume at the intersection. Volumes and delays at this intersection should be monitored during and after construction of the Preferred Alternative.

#### 5.3 Randolph Street Corridor

Table 5-3 summarizes the levels of service for the Randolph Street corridor for the existing conditions, Future No-Build, and Preferred Alternative. All intersections along Randolph Street either have or are expected to have a LOS of D or better for all scenarios.

Table 5-3: Randolph Street Corridor LOS

Intersection	Existing		Future No- Build		Preferred Alternative	
	AM	PM	AM	PM	AM	PM
Randolph/Larned	В	С	В	С	С	С
Randolph/Cadillac Square - S	С	С	С	С	С	D
Randolph/Lafayette	Α	Α	Α	Α	С	С
Randolph/Monroe	В	В	В	В	В	В
Randolph/Gratiot	С	D	С	С	D	D
Randolph/Cadillac Square - N	В	В	В	В	В	В

#### 5.3.1 Randolph Street/Congress Street

Existing conditions show a four-lane cross section along Congress Street, with two travel lanes and two parking lanes between Brush Street and Randolph Street. In order to improve the LOS for the Randolph Street/Congress Street intersection in the Preferred Alternative, three of the four lanes may be needed as travel lanes in the PM peak hour between Brush Street and Randolph Street, keeping only one lane as a parking lane.

#### 5.3.2 Randolph Street/Gratiot Avenue

The initial analysis indicated a LOS E in both the Preferred Alternative AM and PM scenarios at Randolph Street at Gratiot Avenue. The northbound left turn and westbound left turns operate poorly in the AM peak hour and the southbound left, westbound left, and eastbound thru operate poorly in the PM peak hour. The overall intersection LOS would be improved from a LOS E to a LOS D if the northbound left turn would be restricted, as it currently is signed, and the westbound thru lane on Gratiot Avenue is changed to a shared westbound left/thru lane. Following is a more detailed discussion of the intersection by movement.

#### Northbound Randolph Street Left Turn to Gratiot Avenue Westbound

The northbound left turn movement is expected to be a LOS F in the Future No-Build condition. The delay is increased from 90.5 seconds in the AM Future No-Build condition to 172 seconds in the AM Preferred Alternative condition and impacts approximately 185 vehicles. At this intersection, it was noted that 115 vehicles in the AM peak hour were making a left turn, despite the fact that the pavement markings and signs indicate left turns are not allowed. Additional green time is required on the northbound approach to accommodate the non-permitted movement and takes away green time from other approaches at the intersection.

#### **Westbound Gratiot Avenue Left Turn to Randolph Street**

The westbound left turning movement goes from LOS D in the No-Build to LOS F in the Preferred Alternative AM and PM scenario, exhibiting 107 seconds of delay in the AM Build and 114 seconds of delay in the PM Build. This is due to an increase in demand from 440 to 620 vehicles in the AM peak hour and from 200 to 460 vehicles in the PM

peak hour from No-Build to Build, respectively. As indicated earlier, changing the westbound thru lane to a shared westbound left/thru lane will reduce the westbound left delay to 63.6 seconds of delay in the AM Build and 75.2 seconds of delay in the PM Build. The changes to the westbound approach at Gratiot Avenue and Randolph Street implemented in the Build Synchro files and the results are reflected in Table 5-3.

#### **Southbound Broadway Street Left Turn to Gratiot Avenue**

The southbound left turning movement goes from a LOS D in the Future No-Build PM peak hour to a LOS F in the Preferred Alternative PM peak hour scenario, exhibiting 128 seconds of delay. The demand for this movement was slightly increased by 12 vehicles with the Preferred Alternative. However, signal timing adjustments were made to accommodate other movements which decreased the level of service for this movement.

#### **Eastbound Gratiot Avenue**

The eastbound approach contains all movements in one lane and is therefore considered by the full approach and not by turning movement. The approach goes from a LOS D in the Future No-Build PM peak hour to a LOS F in the Preferred Alternative PM peak hour scenario, exhibiting 120 seconds of delay. The demand for this movement was slightly increased by 16 vehicles with the Preferred Alternative. However, signal timing adjustments were made to accommodate other movements which decreased the level of service for this movement. The approach delay is decreased with the capacity improvements for the westbound left turn and restricting the northbound left movement.

#### 5.3.3 Randolph Street/Cadillac Square South

While the intersection is a LOS D with the Preferred Alternative in the PM peak hour, the eastbound right turn movement experiences a LOS E with a delay of 56 seconds and the eastbound left turn movement experiences a LOS F with a delay of 90 seconds. However, this impacts 89 right turning vehicles and 70 left turning vehicles in the PM peak hour, respectively. Given the acceptable LOS for all other movements, and the low volume of impacted vehicles, no change is recommended at this location.

#### 5.4 Beaubien Street Corridor

Table 5-4 summarizes the levels of service for the Beaubien Street corridor for the existing conditions, Future No-Build, and Preferred Alternative. All intersections along Beaubien Street either have or are expected to have a LOS of D or better for all scenarios.

Table 5-4: Beaubien Street Corridor LOS

Intersection	Existing		Future N	No-Build	Preferred Alternative	
	AM	PM	AM	PM	AM	PM
Beaubien/Lafayette	Α	Α	Α	Α	В	В
Beaubien/Monroe	В	Α	В	Α	В	В
Beaubien/Congress	В	Α	В	В	В	В
Beaubien/Larned	В	В	Α	В	Α	D
Beaubien/Gratiot	С	В	С	В	D	С
Beaubien/Fort	В	В	В	В	В	В

#### 5.4.1 Beaubien Street/Gratiot Avenue

There is one intersection in the Preferred Alternative AM Peak Hour that exhibits a LOS F for the westbound left turn from Gratiot Avenue onto Beaubien Street. The LOS F movement can be attributed to the large increase in the westbound left turn volume from the No-Build to the Preferred Alternative. The westbound left turn volume in the No-Build AM peak hour is 224 vehicles and is expected to increase to 470 vehicles with the Preferred Alternative. A potential improvement at the intersection would be to add a protected left turn signal phase for this movement. There is already a left-turn lane for this approach, so adding a left-turn signal head would be a minimal expense and would provide better operations for the westbound left turn as well as the overall intersection.

#### 5.5 Congress Street Corridor

Table 5-5 summarizes the levels of service for the Congress Street corridor for the existing conditions, Future No-Build, and Preferred Alternative.

Table 5-5: Congress Street Corridor LOS

Intersection	Existing		Future N	No-Build	Preferred Alternative	
	AM	PM	AM	PM	AM	PM
Congress/Bates	Α	Α	Α	Α	Α	В
Congress/First	В	F	В	F	С	F
Congress/Griswold	В	В	В	В	В	В
Congress/Shelby	В	D	В	С	В	С
Congress/Washington	В	В	В	В	В	С
Congress/Woodward	Α	Α	Α	Α	Α	В

In the AM peak hour, all intersections along the Congress Street corridor either have or are expected to have a LOS D or better in the Existing conditions, FNB, and the Preferred Alternative. There is one intersection in the PM peak hour in the future FNB and Preferred Alternative that is expected to have a LOS F. The following sections describe the

intersection analysis for those intersections that are expected to have poor LOS for either a movement or approach.

#### 5.5.1 Congress Street/Shelby Street

Currently, Congress Street between Griswold Street and Washington Boulevard is one-way westbound and has a three-lane cross section, with one travel lane and two parking lanes. During the PM peak period, parking is supposed to be restricted in this section, however, this restriction is often ignored. In order to improve the LOS in the PM peak hour for Congress Street at Shelby Street, two of the three lanes would need to be travel lanes and the parking restriction needs to be enforced.

#### 5.5.2 Congress Street/First Street

In the PM peak hour, Congress Street at First Street is a LOS F in the existing, FNB, and the Preferred Alternative. This can be attributed to the large westbound through volume utilizing the left most lane on Congress Street west of First Street. Even though there are two lanes, only the left most lane goes to northbound M-10. The demand for the westbound through movement is expected to grow from 975 vehicles to 1,340 vehicles in the PM peak as a result of the Preferred Alternative. The intersection delay will reduce significantly if the traffic signal is put into flash and the westbound approach is able to flow unimpeded through the intersection. The northbound and southbound approaches would receive a flashing red traffic signal basically converting the intersection into a two-way stop. The intersection delay in the Preferred Alternative in the PM peak hour would be reduced to 74.9 seconds. The improvements were not implemented into the Synchro models but could be implemented in the future to reduce the delay and congestion at the intersection.

#### 5.6 Woodward Avenue Corridor

Table 5-6 summarizes the levels of service for the Woodward Avenue corridor for the existing conditions, Future No-Build, and Preferred Alternative.

Table 5-6: Woodward Avenue Corridor LOS

Intersection	Existing		Future N	lo-Build	Preferred Alternative	
	AM	PM	AM	PM	AM	PM
Woodward/Montcalm	В	Α	В	Α	Α	Α
Woodward/Elizabeth	Α	Α	Α	В	Α	В
Woodward/Adams	Α	В	Α	В	Α	С
Woodward/Park	Α	Α	В	Α	Α	В
Woodward/John R	Α	В	Α	В	Α	В
Woodward/Grand River	В	В	В	В	Α	В
Woodward/Gratiot	Α	Α	Α	Α	В	В

All intersections along the Woodward Avenue corridor either have or are expected to have a LOS D or better for the existing conditions, FNB, and Preferred Alternative. The signal timings implemented in Synchro accounted for the pedestrian phases and the appropriate clearance time based on the distance of the cross-walk. No additional improvements or modifications are recommended for this corridor.

# 6 HCS Development Methodology

While Synchro was utilized for the intersection analysis, the Highway Capacity Software (HCS) was utilized for the freeway analysis. Existing AM and PM peak hour HCS models were created to use as a baseline for the Future No-Build and Preferred Alternative scenarios. Traffic counts were obtained as discussed in Section 3. The Future No-Build and Preferred Alternative scenario used the same geometry as the existing conditions. The Future No-Build scenario used a growth factor of 0.5% per year compounded from year 2017 to 2040. The Preferred Alternative considered rerouting as a result of the I-375 project in the traffic forecasts, as documented in the I-375 Traffic Forecasting Methodology Technical Memorandum.

#### 7 HCS Results

The HCS analysis was conducted on the study area shown in Figure 2. Results of the HCS analysis can be found in Table 7-1. The LOS and density results for the 18 segments along M-10 are provided in Appendix C.

Figure 2: HCS Segments

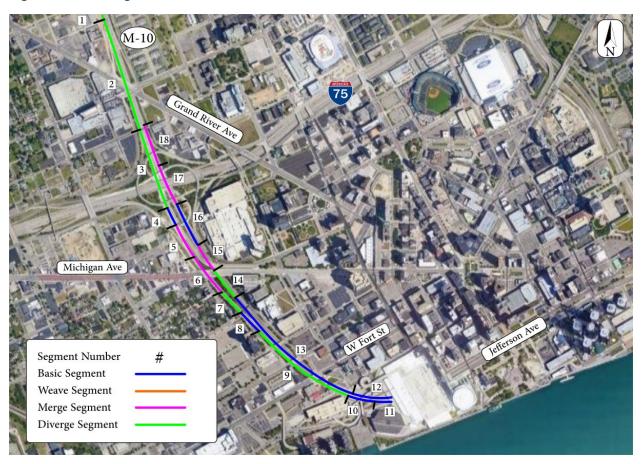


Table 7-1: M-10 Freeway Segment LOS Results

			os					
ID	Dir	Segment		AN	Л		PM	
			Ex*	FNB	Preferred	Ex*	FNB	Preferred
1	SB	Forest On-ramp to Grand River Off-ramp	F	F	F	С	D	D
2	SB	Grand River Off-ramp to I-75 Off-ramp	F	F	F	С	С	С
3	SB	I-75 Off-ramp to SB I-75 On-ramp	D	D	D	В	В	В
4	SB	SB I-75 On-ramp merge	В	В	С	Α	Α	В
5	SB	SB I-75 On-ramp to NB I-75 On-ramp	В	С	С	В	В	В
6	SB	NB I-75 On-ramp merge	D	Е	E	В	В	С
7	SB	NB I-75 On-ramp to Howard Off-ramp	D	Е	Е	В	В	С
8	SB	Howard Off-ramp to Abbott	С	С	С	Α	Α	В
9	SB	Abbott to WB Jefferson Off- ramp diverge	С	С	С	В	В	В
10	SB	WB Jefferson Off-ramp to Larned Off-ramp	В	С	С	Α	Α	В
11	SB	Larned Off-ramp to EB Jefferson	В	В	В	Α	Α	Α
12	NB	WB Jefferson to Congress On- ramp	Α	Α	Α	С	С	С
13	NB	Congress On-ramp to Abbott On-ramp	Α	Α	Α	С	С	С
14	NB	Abbott On-ramp merge	Α	Α	В	D	D	Е
15	NB	Abbot On-ramp to I-75 Off- ramp	Α	Α	В	F	F	F
16	NB	I-75 Off-ramp to NB I-75 On- ramp	Α	Α	Α	В	В	С
17	NB	NB I-75 On-ramp merge	В	В	В	В	С	С
18	NB	NB I-75 On-ramp to SB I-75 On-ramp	В	С	С	С	С	С

\*Ex = Existing Conditions

The notable volume increase that triggered an additional analysis was observed in both the AM and PM peak hours. Volumes increased by up to 725 vehicles in the PM peak hour. As the LOS results show, there is a sufficient amount of capacity in the PM peak hour to support this increase. In the PM peak hour, northbound M-10 at Abbott Street (#14), shows a change from a LOS D in the FNB while the Preferred Alternative is expected to have a LOS E. The density for the freeway segment increased from 33.8 to 37.4 passenger cars per mile per lane between the Future No-Build and the Preferred Alternative, respectively. The density results show a minor change in the freeway

operations and no recommendations are provided for the segment. The poor LOS segments in the AM and PM peak hours along M-10 are consistent between the Future No-Build and Preferred Alternative. No mitigation measures are recommended.

#### 8 Recommendations

Recommendations made as a result of this analysis are separated into two categories. Geometric improvements that were more simplistic and helped achieve an intersection LOS D or better are referred to as "Intersection LOS Improvements" and were nearly all assumed as part of the analysis. Other improvements that benefitted individual movements at intersections where the intersection as a whole was LOS D or better are referred to as "Movement LOS Improvements". These were often more complex and were not included in the analysis. This section summarizes the previously discussed improvements into one of those two categories. It should be noted that recommendations are based on future traffic volumes and the expected shifts in travel patterns based on the Preferred Alternative and the DTA models. Travel patterns may end up being different during and after construction of the Preferred Alternative. Some of these recommendations could be implemented on a "wait and see" condition to see if the traffic materializes. An additional traffic analysis could also be conducted immediately prior to construction or immediately after to evaluate conditions again.

#### 8.1 Intersection LOS Improvements

Improvements that will improve all intersections to LOS D or better, and were assumed in the analysis:

- Congress Street between Randolph Street and Beaubien Street one parking lane converted to a driving lane, changing the total number of driving lanes from two lanes to three lanes and leaving one parking lane.
- Congress Street between Griswold Street and Washington Boulevard one parking lane converted to a driving lane, changing the total number of driving lanes from one lane to two lanes and leaving one parking lane on westbound Congress Street.
- Mack Avenue at Russell Street Installed a dedicated left turn bay for the northbound left movement to improve the level of service for the northbound approach. Added a small storage lane for the eastbound right turn at the intersection to improve operations of the overall intersection.
- Randolph Street at Gratiot Avenue Enforce the no northbound left turns from Randolph Street to Gratiot Avenue, as well as change the westbound thru lane to a shared westbound left/thru lane on Gratiot Avenue.
- **Brush Street at Gratiot Avenue** Install a westbound left turn lane on Gratiot Avenue and provide a protective/permissive signal phase.

The one location that still may have a LOS F is at **Congress Street at First Street**. The intersection delay will reduce significantly if the traffic signal is put into flash and the westbound approach is able to flow unimpeded through the intersection. The northbound

and southbound approaches would receive a flashing red traffic signal basically converting the intersection into a two-way stop. The intersection delay in the PM Build would be reduced to 74.9 seconds. The improvements were not implemented into the Synchro models but could be implemented in the future to reduce the delay and congestion at the intersection.

#### 8.2 Movement LOS Improvements

The following improvements were **not** assumed for the analysis and may require additional analysis or consideration. Without these improvements, the intersections will all operate at LOS D or better, but there may be some individual movements that operate below LOS E or worse. The locations include:

#### Brush Street at Larned Street

Restrict parking on southbound Brush Street to provide an additional travel lane

#### Beaubien Street at Gratiot Avenue

 Add a southbound protected left turn signal phase for Gratiot Avenue if there are excessive delays.

#### Beaubien Street at Larned Street

 Add a southbound protected left turn signal phase for Beaubien Street at Larned Street if there are excessive delays.

#### Woodward Avenue at Adams Avenue

The northbound left turn in the Preferred Alternative in the PM peak hour is a LOS E, with a volume of 334 vehicles. The northbound left turn is currently permissive but could be upgraded in the future to protected/permissive in order to improve the LOS of the northbound approach.

# Appendix A – TDM Volume Maps



Appendix B – Synchro Analysis Results					

#### I-375 Expanded Study Area - Synchro Results

	AM Peak H	our - Existing	PM Peak Ho	ur - Existing	AM Peak Ho	our - No Build	PM Peak Ho	our - No Build	AM Peak	Hour - Build	PM Peak I	Hour - Build
Intersection	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay
1st & Congress	В	16.5	F	344	В	15.2	F	314.6	С	23.5	F	487.8
Washington & Congress	В	16.1	В	13.5	В	18.6	В	14.9	В	19.8	С	25.1
Shelby & Congress	В	15	D	37.8	В	16.5	С	27	В	14.9	С	22.7
Griswold & Congress	В	11.3	В	10.4	В	11.5	В	10.8	В	11.7	В	12.7
Woodward & Congress	Α	9.3	Α	8.1	A	9.2	Α	8.4	Α	9.3	В	11
Bates & Congress	Α	5.5	Α	7.9	A	5.6	Α	8.5	A	5.3	В	12.7
Randolph & Gratiot	С	28.5	D	50.6	С	33.6	С	31	D	53.4	D	48.6
Randolph & Monroe	В	11.6	В	14.1	В	11.2	В	14.5	В	13.3	В	15.7
Randolph & Lafayette	Α	7.9	Α	8	A	8.9	Α	7.9	С	22	С	24.2
Randolph & Cadillac Sq - N	В	12.1	В	11.9	В	11.6	В	12.5	В	18.9	В	19.9
Randolph & Cadillac Sq - S	С	23.9	С	24.9	С	27.4	С	25.6	С	32.5	D	47.9
Randolph & Larned	В	19.8	С	20.8	В	19.5	С	21.4	С	22.4	С	26.2
Brush & Adams	Α	7.6	Α	7.7	A	7.6	Α	7.9	Α	9.9	В	11.1
Brush & Beacon	Α	6.1	Α	6.8	A	6.2	Α	6.9	A	8.2	Α	8.2
Brush & Madison	В	11.8	В	15.9	В	11.9	В	15.4	В	12.4	В	15.9
Brush & Gratiot	В	10.7	В	14.6	В	11.6	В	14.8	D	49.5	С	33.5
Brush & Monroe	В	13.7	В	14	В	14.3	В	14.4	В	11.6	В	14.3
Brush & Congress	В	13.8	В	11.1	В	13.9	В	11.6	В	12.8	С	34.7
Brush & Larned	В	15.6	В	11.5	В	16.5	В	12.4	С	23.1	D	47.9
Beaubien & Gratiot	С	24.1	В	17.1	С	25.7	В	17.4	D	36.2	С	23.5
Beaubien & Monroe	В	11.7	Α	8.3	В	15	Α	8.8	В	13.6	В	14.7
Beaubien & Lafayette	Α	9.8	Α	6.1	A	10	Α	6.4	В	13.1	В	12
Beaubien & Fort	В	14.4	В	14.7	В	14.5	В	14.1	В	12.5	В	16.9
Beaubien & Congress	В	10.1	Α	10	В	12.2	В	11	В	11.6	В	15.4
Beaubien & Larned	В	10.3	В	12.3	A	9.8	В	13.3	A	9.4	D	53.4
Mack & Russell	Α	8.8	Α	8.5	В	13.3	Α	9.5	В	15.5	В	14.9
Mack & St. Aubin	В	16.3	В	13.4	В	20	В	15.4	D	54.3	D	39.2
Woodward & Montcalm	В	10.2	Α	6.4	В	10.3	Α	6.5	A	9.2	Α	6.8
Woodward & Elizabeth St	Α	6.7	Α	9.9	A	6.5	В	10.1	A	4.1	В	11.7
Woodward & Adams Ave	Α	7.2	В	13.7	Α	7.1	В	14.5	Α	9.2	С	23.4
Woodward & Park Ave	Α	9.8	Α	8.5	В	11.1	Α	8.6	Α	8	В	11.6
Woodward & John R St.	Α	4.5	В	11.5	Α	4.8	В	12.1	Α	3.8	В	17.3
Woodward & Grand River Ave	В	11.8	В	14.5	В	12.9	В	14.9	Α	10	В	17.1
Woodward & Gratiot Ave	Α	9.8	Α	6.1	A	10	Α	6.4	В	13.1	В	12

								Build S	ynchro Analysis (	AM & PM) -	I-375 Expande	d Study Area									
Intersection	Traffic Control	Peak Hour			Eas	tbound			Westb		f Service per N	lovement by	Approach Northi	ound			South	bound		Intersection Delay	Intersection LOS
			Lanes	U-turn	LT	TH	RT	U-turn	LT <1	TH 1>	RT 0	U-turn	LT <1	TH	RT	U-turn	LT	TH 1>	RT 0>		
		AM	Volume Delay LOS	-				-	95 9.5 A	285 24.8 C	181	-	4 16 B	-		-		20 34.2 C	36	23.5	С
Congress & 1st Street	Signalized	PM	95th Queue Approach LOS Volume Delay LOS	:	-	<u>:</u>	:	-	1 10 A	247 1340 490.6	66	-	79 20.6 C	22 16.4 B		:	:	52 C 0 597.1	410	487.8	F
		FW	95th Queue Approach LOS Lanes	-		<u> </u>	-		m43	247	0	-	55 E	22			-	#539 F 2>	0	407.0	r
Congress & Washington	Signalized	АМ	Volume Delay LOS 95th Queue Approach LOS	-	-		-	-	31 - -	275 14.7 B 61	62	-	398 32.3 C #256	358 11 B 74	-	-	-	220 20.4 C 80	67	19.8	В
		PM	Volume Delay LOS 95th Queue Approach LOS		- - -		- - -	-	16 - -	881 30.2 C 61	107	-	197 19.8 B 91	225 10.4 B 47	-	-		253 22.5 C 136	182	25.1	С
		AM	Volume Delay LOS 95th Queue		-	-	-	-	1 -	<2> 310 10.9 B 74	30	-	0 57 -	<1 66 22.8 C	-	-	-	1> 80 17.4 B	30	14.9	В
Congress & Shelby	Signalized	PM	Approach LOS Volume Delay LOS	-	- - - -	<u> </u>	-	-	16	895 23.6 C	48	-	56	33 20.3 C	 - - -	-	-	51 15.9 B	23	22.7	С
			95th Queue Approach LOS Lanes Volume Delay	-			:	-	0 42	<2 248 11.3	1 95 3.5	-	0 46	<2 223 15.3	-	-	-	45 B 2> 248 11.7	0 70	11.7	В
Congress & Griswold	Signalized	AM	LOS 95th Queue Approach LOS Volume Delay				-		20	838 12.7	A 12 222 2.5		57	B 73 168 15.8				B 71 B 373 16.2	84		
		PM	LOS 95th Queue Approach LOS Lanes	-		-	-	-		B 52 3	A 12	-	1 20	B 66				B 120 B 3>	0	12.7	В
Congress & Woodward	Signalized	AM	Volume Delay LOS 95th Queue Approach LOS						22 - - -		50 - -		39 15.9 B 27				-	234 8.8 A 38	78	9.3	A
		PM	Volume Delay LOS 95th Queue Approach LOS				-		50 - -		132	-	54 14 B 35	116 11.4 B 20		-	:	192 11 B 36	52	11	В
Congress & Bates	Signalized	AM	Volume Delay LOS 95th Queue Approach LOS	-			-	-	- - - -	3> 280 6.7 A 36	74	-	-	-		-		- - - - -	1 92 0.6 A	5.3	А
y and a total	0	PM	Volume Delay LOS 95th Queue Approach LOS	-	:		:	-	: : :	1086 12.4 B 36	128	-	-	-	:	-		- - - - - - -	100 16.4 B	12.7	В
		AM	Volume Delay LOS 95th Queue	-	<1> 2 15.5 B 0		:	-	1 616 63.6 <b>E</b> 497	<2> 817 68.4 <b>E</b> 506	0 37 - -		<1 184 54.5 D 217	- - - -	1 235 9.3 A 39	-	1 75 58.7 <b>E</b> 108	2> 180 37.6 <b>D</b> 132	0	53.4	D
Randolph & Gratiot	Signalized	PM	Approach LOS  Volume  Delay  LOS  101st Queue	-	139 74.1 E	B	:		457 75.2 E 497	230 67.6 E 506	:	-	260 37.9 D		506 9.5 A 168	-	118 71.4 E 112			48.6	D
		AM	Approach LOS  Lanes  Volume  Delay  LOS		1 118 38.1 D	2> 61 25 C	1 49 11.4 B	-			-	-	0 51	<2 296 9.3 A	1 35 3.2 A	-	0 68 -	<2> 591 13.9 8	1 280 3.4 A	13.3	В
Randolph & Monroe	Signalized	PM	95th Queue Approach LOS Volume Delay LOS		205 45.1 D	39 C 87 32 C	31 11.7 B						129	579 13.1 B	58 3.6 A		41	165 B 386 10 B	157 2.3 A	15.7	В
			95th Queue Approach LOS Lanes Volume Delay			39 D -		-	0 37	<1> 106 47.3	0 122	-	0 8	<2> 281 11.8	0 51			100 A <2> 441 14.8	0 89	22	С
Randolph & Lafayette	Signalized	AM	LOS 95th Queue Approach LOS Volume Delay			1 : : : : : : : : : : : : : : : : : : :	:		75	90 54.8	152		13	561 15.6	1		-	B 140 B 370 12.5	83		_
		PM	LOS 95th Queue Approach LOS Lanes Volume	-		-	-		- - - -	#265		-	<1 82	2 457	-	-		B 120 B 2 264	1 25	24.2	С
Randolph & Cadillac Sq - N	Signalized	AM	Delay LOS 96th Queue Approach LOS Volume				:		:	-	:	-	73.8 E m74 E	7.2 A m41			:	23.2 C 102 C	6.9 A	18.9	В
		PM	Delay LOS 96th Queue Approach LOS	-			-	-	:		-	-	117.5 F m89	5.3 A m43		-	-	21.4 C 117	7.9 A	19.9	В

								Build Sy	nchro Analysis (/	AM & PM) -	I-375 Expande	ed Study Area									
Intersection	Traffic Control	Peak Hour			Eastbo	ound			Westbo	ound	f Service per N		North	bound			South	hbound		Intersection Delay	Intersection LOS
Randolph & Cadillac Sq -	Signalized	AM	Lanes Volume Delay LOS 95th Queue Approach LOS	U-turn 1 41 55.1 E 61	.T	TH 2> 60 49.4 D 44		U-turn	26 C	7H <3> 487 27.5 C 143		U-turn	- - - -	2 406 50.6 D #245		U-turn	- - - -	3> 164 5.1 A 16	0 169	32.5	С
s		PM	Volume Delay LOS 95th Queue Approach LOS	70 89.7 <b>F</b> 61	E	89 55.7 <b>E</b> 44		-	190 - - - E	687 60 <b>E</b> 143	- - - -	- - - -	- - - -	543 50.4 D 181	- - - -			350 5.3 A 23	76	47.9	D
Randolph & Larned	Signalized	AM	Lanes Volume Delay LOS 95th Queue Approach LOS		1 108 23.9 C 100		1 47 22.2 C 51	-	· · · · · · · · · · · · · · · · · · ·	- - - - -	- - - - -	- - - -	- - - - -	2> 474 23.5 C 274	245	-		133 17.2 B 116	: : :	22.4	С
		PM	Volume Delay LOS 95th Queue Approach LOS Lanes		165 26.6 C 100	543 26 C 130	259 36.5 D 51	-	· .		-	-	1	431 20.8 C 152	80	-	52	490 28.8 C m243 C	0	26.2	С
Brush & Adams	Signalized	AM	Volume Delay LOS 95th Queue Approach LOS Volume	: : :	6 8.5 A 4 A	- - - -	19 4.3 A 7	-	: : : :	-	: : : :		54	124 5.8 A 20 A		-	:	238 12.5 B 90 B	41	9.9	A
		PM	Delay LOS 95th Queue Approach LOS Lanes		8.5 A 4	- - - -	5 A 7	-		-	1	-	7.7 A 18	12.6 B 83 B	0	-	-	8.7 A 36 A	-	11.1	В
Brush & Beacon	Signalized	AM	Volume Delay LOS 95th Queue Approach LOS Volume		:			- - - -			8 0 A 0			147 9.5 A 59 A	28			204 7.7 A 40 A		8.2	A
		PM	Delay LOS 95th Queue Approach LOS Lanes		-	· · · · · · · · · · · · · · · · · · ·			- - - A	<2>	0.1 A 0	-	1	10.7 B 69 B	0	-	1	5.5 A 17 A		8.2	А
Brush & Madison	Signalized	AM	Volume Delay LOS 95th Queue Approach LOS Volume	-	22 - - - - - - - - 31	212 14 B 60	1		1	345 13.7 B 93	49		96 13.8 B 55	103 8.5 A 80 A	103		56 12.4 B 34	121 11.2 B 79 B	46	12.4	В
		РМ	Delay LOS 95th Queue Approach LOS Lanes	1	2>	17.3 B 60	-	-	- - B	12.5 B 93	1	-	14.3 B 70	17.4 B 157 B	0		18.5 B 44	4.9 A 28	-	15.9	В
Brush & Gratiot	Signalized	AM	Volume Delay LOS 95th Queue Approach LOS Volume	36 38.9 D 55	211 18.4 B 89 C	:	-	:	211 12.3 B 111 E	1367 71.2 E 557	7.6 A 87		40	74 53.7 D 226 D	113	-		C 121	51	49.5	D
		PM	Delay LOS 95th Queue Approach LOS Lanes	52.4 D 55	39.2 D 89	<2>		-	- - - B	19.6 B 557	3.5 A 87	-	-	54 D #402	0	-	0	19.4 B 122 B	-	33.5	С
Brush & Monroe	Signalized	AM	Volume Delay LOS 95th Queue Approach LOS Volume		20 - - - - - - - - - - - - - - - - - - -	223	31	-	: : :	-	:	-	-	135 4 A 43 A 309	186		91	251 5.6 A 103 A		11.6	В
		PM	Delay LOS 95th Queue Approach LOS Lanes		- - - C	29.8 C 66		-	0 67	<3> 613	0 42	-	0 111	9.6 A 218 A <1 195	:	-		5.7 A 80 A 1> 241	0 41	14.3	В
Brush & Congress	Signalized	AM	Delay LOS 95th Queue Approach LOS Volume		-	:	-		B 209	14.5 B 66	109	-	-	12 B 147 B			-	8.6 A 102 A	101	12.8	В
		PM	Delay LOS 95th Queue Approach LOS Lanes Volume		0 89	<3> 445	0 41		- - - D	46.4 D 66				8.9 A m79 A 1> 257	0	-	0 126	13.5 B 98 B <1		34.7	С
Brush & Larned	Signalized	AM	Delay LOS 95th Queue Approach LOS Volume		- - - - - - - - - - - - - - - - - - -	22.8 C 122	144	-		:	:	-	-	13.1 B 189 B	55			37.3 D 124 D		23.1	С
		PM	Delay LOS 95th Queue Approach LOS Lanes Volume	1 1	2> 195	36.1 D 122		-	1 470	2 1449	1 341		1 41	11.7 B 105 B	0 61	-	0 37	94.9 F m#337 F <2> 64	0 362	47.9	D
Beaubien & Gratiot	Signalized	АМ	Delay LOS 95th Queue Approach LOS Volume	13 B 3	12.5 B 57 B 844 28	:	-		67 E #548 D	35.1 D #729	12.3 B 179	-	31 C 56	10.3 B 49 B	201		-	36.9 D 150 D	79	36.2	D
		PM	Delay LOS 95th Queue Approach LOS Lanes Volume		28 C 57 C	2> 106	0 74		50.4 D #548	C #729			B 39	B 190 B 1> 61	0 1		0 90	C <1 264		23.5	С
Beaubien & Monroe	Signalized	AM	Delay LOS 95th Queue Approach LOS Volume Delay		A	9.3 A 55 307	107					-	-	6.3 A 17 A 91 2.7	1		139	17.8 B 194 B 182 19.7		13.6	В
		PM	LOS 95th Queue Approach LOS	-		B 55		-		-				2.7 A 4		-		19.7 B 176		14.7	В

								Build Sy	nchro Analysis	(AM & PM) -	I-375 Expande	d Study Area									
Intersection	Traffic Control	Peak Hour			Eastb				Westl	bound	f Service per N		North					nbound		Intersection Delay	Intersection LOS
		AM	Lanes Volume Delay LOS	U-turn - -	0 31	31 11.4 B	0 1	U-turn - -	1 161 13.4 B	2> 484 12.9 B	0 1	U-turn - -	0 1	1 14.5	1 122 6.3 A	U-turn - -	0 69	198 16.4 B	73	13.1	В
Beaubien & Lafayette	Signalized		95th Queue Approach LOS Volume Delay		71	37 3 11 10.4	21	-	85 128 13.8	104 3 325 11.8	21	-	1	m3 A 1 9.5	0 565 7.3	:	123	223 B 125 20.7	65		_
		PM	LOS 95th Queue Approach LOS Lanes Volume	-	1>	37 3 -	0 25	-	8 85 E	B 104 B	-	-	-	A m1 A <2 180	A 80	-	-	C 185 C 2> 265	-	12	В
Beaubien & Fort	Signalized	AM	Delay LOS 95th Queue Approach LOS Volume	-	9.9 A 31	- - - -	166	-	: : :	- - - -	:	-	- - -	13.6 B 50				12.4 B 54 B		12.5	В
		PM	Delay LOS 95th Queue Approach LOS	-	21.7 C 31		-	-		-	-	-		13.2 B 102	-	-		15.6 B 80	-	16.9	В
		AM	Volume Delay LOS 95th Queue	- - - -	-	- - - - -	-	-	56	<4> 618 15.5 B 90	0 46 - -	-	1 68 10.7 B 26	2 189 9.7 A 30	-		-	2> 179 3.6 A 0	86	11.6	В
Beaubien & Congress	Signalized	PM	Approach LOS  Volume  Delay  LOS  95th Queue	- - - -		·	- - - -	-	99 - -	674 16 B 90	215	-	167 24.4 C #64	136 9.6 A 24	-	:		232 12.6 B 116	242	15.4	В
		AM	Approach LOS  Lanes  Volume  Delay  LOS  95th Queue	- - - -	1 52 8.2 A m20	2 491 9.8 A m84	1 38 2.6 A m3	-	- - -	-	- - - -	-	- - - -	2> 225 10.6 B 63	0 128		1 51 9.1 A 18	2 116 7.6 A 17	- - -	9.4	А
Beaubien & Larned	Signalized	PM	Approach LOS  Volume  Delay  LOS  95th Queue		85 23.1 C m20		29 10.8 B m3	-		· · · · · · · · · · · · · · · · · · ·	-	-	- - -		185			70 7.6 A	-	53.4	D
		AM	Approach LOS Lanes Volume Delay LOS	-	0 3		0 161		0 87	<3> 1237 18.2 B	0 8		0 229		0 51			10 (1> (1) (1) (1) (1) (1) (1) (1) (1) (1) (1)		15.5	В
Russell & Mack	Signalized	PM	95th Queue Approach LOS Volume Delay LOS	•	3	108	212	-	68 -	242	11	-	393	32	128	-	12	5 B 3 15.4 B	8	14.9	В
			95th Queue Approach LOS Lanes Volume Delay	-	0 18	108 3 <1 291 21.1	1 245 9.7	-	0 20	242 3 <1> 869 71.8	0 12	-	1 350 77	103 2 1> 50 33.4	0 23	-	0 6	6 B <1> 115 68.6	0 52	54.3	E
Mack & St. Aubin	Signalized	AM	LOS 95th Queue Approach LOS Volume Delay	-	22	C 227	A 70 67 6	-	7	#993	8	-	#318 #310 50.3	C 77	20		18	E 173 E 48 28.6	42		
		PM	LOS 95th Queue Approach LOS Lanes	-	0	D 227	A 70 0	-	0	#993 (1>	0	-	D 225	C 74	0		-	C 125 C 3>	0	39.2	D
Woodward & Montcalm	Signalized	AM	Volume Delay LOS 95th Queue Approach LOS Volume	-	12	14.2 B 8	26	-	10 - - - - 28	16 14.5 B 22 3	12 - - - - 52	-		349 6 A 32 A	7		-	1034 10.1 B 135 B	51	9.2	A
		PM	Delay LOS 95th Queue Approach LOS	-	-	20.6 C 8	0	-	- (	32.7 C 22	-		:	2.6 A 28	0		-	9.2 A 101	0	6.8	A
Woodward &	Signalized	АМ	Volume Delay LOS 95th Queue Approach LOS		4	<1> 52 14 B 40	17		1	7 12.9 B 10	3	-		3> 372 4.5 A 24	22		-	3> 1042 2.7 A 16	15	4.1	А
Elizabeth St	- go salitiAbaM	PM	Volume Delay LOS 95th Queue Approach LOS	-	19	6 17.7 B 40	20	-	15	68 32.4 C 10	26	-	- - - -	1187 11 B 194	53	-		592 7.7 A 64	36	11.7	В
		AM	Lanes Volume Delay LOS 95th Queue		-	- - - - -	-	-	55	<3> 102 7.6 A 28	0 114	-	1 37 27.5 C 28	2 282 10.1 B 51		-	- - - -	2 931 10 A 58	1 137 1.2 A 1	9.2	А
Woodward & Adams	Signalized	PM	Approach LOS  Volume  Delay  LOS  95th Queue				- - - -	-	48	315 37.3 D 28	338	-	334 58.9 E 354	679 7.7 A 105		-		540 3.4 A 87	74 1.1 A 0	23.4	С
		AM	Approach LOS Lanes Volume Delay LOS		76	<2 292 21.7 C	1 54 6 A	-	- - - -	· · · · · · · · · · · · · · · · · · ·			- - -	2> 241 7 A	0 75	-	1 313 7.4 A	A 2 663 0.9 A	-	8	A
Woodward & Park	Signalized	PM	95th Queue Approach LOS Volume Delay LOS		97	105 3 157 32.7 C	89 5.7 A	-					- - - -	562 10.6 B	76		85 6.6 A	5 A 619 5.7 A		11.6	В
			95th Queue Approach LOS Lanes Volume Delay		-	105			0 19	<2> 33 14.4	0 10 -	-	0 221	2> 123 7.1	:		m24	64 A 2> 603 1.2	0 120	3.8	A
Woodward & John R	Signalized	AM	LOS 95th Queue Approach LOS Volume Delay				-		89	157 47.1	92		749	70 12.7	:		-	A 10 A 382 1.3	111	47.0	
		PM	LOS 95th Queue Approach LOS						-	D 21			-	B 350				A 0		17.3	В

Build Synchro Analysis (AM & PM) - 1-375 Expanded Study Area																				
									Level o	f Service per I	Movement by	Approach								
Traffic Control	Peak Hour			Eastb	ound			West	bound			North	bound			South	bound		Intersection Delay	Intersection LOS
			U-turn	LT	TH	RT	U-turn	LT	TH	RT	U-turn	LT	TH	RT	U-turn	LT	TH	RT		
		Lanes	-	0	<3>	0	-	-	-		-	-	2>	0	-	0	<2	-		
		Volume	-	121		191							217	28		163		-		А
		Delay	-	-	18.5	-					-		5				2.2	-	10	
Signalized	AM	LOS	-	-	В	-					-		A				A	-		
		95th Queue	-		128	-	-	-	-				27	-			11	-		
									-				A							
			-	188		89					-			66		48	415	-	17.1	В
			-		32	-					-		11.9				7	-		
	PM		-		С	-					-		В				A	-		
			-			-	-	-	-	•		-		-		-	75	-		
			С				-					_					Α			
			-		-		-				-	_		-	-					
												17								
																			9.2	A
	AW																			
err			-	-	-	-	-			-	-	-	40	-		-	59	-		
Signalized										200		70	A02				404	00		1
												/0							1	
	DAA																		8.1	Α
																			0.1	^
																		_		
		AM Signalized PM AM	Lanes  Volume Delay AM LOS S5th Qurue Approach LOS Volume LOS Lanes Approach LOS Lanes Volume Approach LOS Lanes Volume Approach LOS Lanes Volume Delay AM LOS S5th Qurue Approach LOS Volume Delay AM LOS S5th Qurue LOS S5th Qurue LOS S5th Qurue LOS S5th Qurue LOS S6th Qurue Delay LOS S6th Qurue LOS S6th Qurue LOS S6th Qurue LOS S6th Qurue Delay LOS S6th Qurue LOS S6th Qurue Delay	Lanes	Lanes	Lahes	Lanes	Value	Value	Traffic Control   Peak Hour	Fair   Fair	Feak Hour	Feak Houring   Feak	Feak Hour   Feak	Peak Hour   Feathbur   Feathbu	Fee   Fee	Feak Hour   Feak			

Appendix C – HCS Analysis Results													

#### I-375 Expanded Study Area - HCS Analysis

						Α	М			PM							
ID	Facility	Location	Analysis Type	E)	( AM	FNI	В АМ	Bui	ld AM	E)	PM	FN	В РМ	Build PM			
				LOS	Density	LOS	Density										
1	M-10 SB	Grand River Ave	Ramp - Diverge	F	46.2	F	53.9	F	55.4	С	26.0	D	28.2	D	29.1		
2	M-10 SB	I-75	Ramp - Diverge	F	42.8	F	46.1	F	47.9	С	22.5	С	24.6	С	25.4		
3	M-10 SB	Bagley St.	Ramp - Diverge	D	29.4	D	32.0	D	33.4	В	15.5	В	16.9	В	19.4		
4	M-10 SB	WB I-75 On - SB Bagley St. Off	Freeway Segment	В	15.4	В	17.2	С	18.5	Α	8.0	Α	9.0	В	11.1		
5	M-10 SB	I-75	Ramp - Merge	В	19.3	С	21.1	С	21.9	В	11.2	В	12.1	В	14.2		
6	M-10 SB	I-75	Ramp - Merge	D	32.5	Е	35.8	E	37.1	В	15.4	В	16.7	С	20.6		
7	M-10 SB	Howard St.	Ramp - Diverge	D	33.2	E	36.0	E	36.7	В	17.4	В	18.9	С	22.5		
8	M-10 SB	SB Howard St. Off - Abbott St.	Freeway Segment	С	18.5	С	20.7	С	22.6	Α	7.3	Α	8.1	В	12.0		
9	M-10 SB	Jefferson Ave.	Ramp - Diverge	С	24.0	С	26.1	С	27.7	В	12.1	В	13.1	В	17.4		
10	M-10 SB	SB Jefferson Ave Off - EB Larned St. Off	Freeway Segment	В	17.9	С	19.9	С	21.9	Α	6.5	Α	7.3	В	11.2		
11	M-10 SB	EB Larned St. Off - EB Jefferson Ave	Freeway Segment	В	13.7	В	15.3	В	17.9	Α	4.7	Α	5.3	Α	10.2		
12	M-10 NB	WB Jefferson Ave - WB Congress St. On	Freeway Segment	Α	0.9	Α	1.0	Α	3.9	С	18.6	С	20.7	С	24.4		
13	M-10 NB	WB Congress St. On - WB Abbott St. On	Freeway Segment	Α	1.2	Α	1.3	Α	3.5	С	18.9	С	21.1	С	25.1		
14	M-10 NB	Abbott St.	Ramp - Merge	Α	8.8	Α	9.4	В	11.7	D	30.7	D	33.8	E	37.4		
15	M-10 NB	I-75	Ramp - Diverge	Α	8.1	Α	8.7	В	11.8	F	34.6	F	37.6	F	41.1		
16	M-10 NB	I-75 Off - NB Bagley St. On	Freeway Segment	Α	2.2	Α	2.4	Α	4.4	В	14.6	В	16.3	С	18.8		
17	M-10 NB	Bagley St.	Ramp - Merge	В	11.2	В	12.4	В	13.8	В	19.3	С	21.4	С	23.3		
18	M-10 NB	I-75	Ramp - Merge	В	19.9	С	21.9	С	22.2	С	22.2	С	24.4	С	25.6		

Density - passenger cars per mile per lane